

CYLINDER FOUNDATION FOR LARAMIE COUNTY COMMUNITY COLLEGE

NORDTANK 15M ROTOR WIND TURBINE

DIRECT EMBED PIER

LARAMIE, WY

74' TUBE TOWER

DATE: 11-1-05

JOB NO. 05-066

Steel / Concrete / Bolts

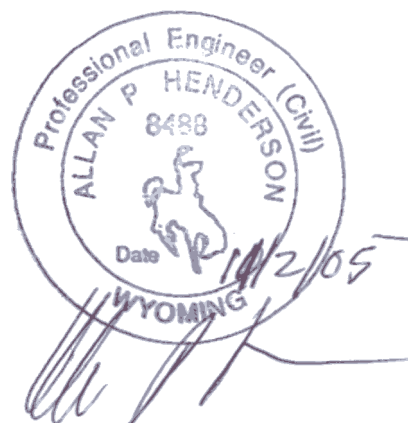
Design Wind Velocity 100 MPH (BASE WIND SPEED)		
Turbine : Nordtank 15M Rotor		
Tower: 74 Feet		
CMP Inside Diameter (Di) =	0	FT.
Tube Outside Diameter (Do) =	8.00	FT.
Inside Bolt Diameter (Bi) =	0	FT.
Outside Bolt Diameter (Bo) =	6.2927	FT.
Inside. Bse. Flng. Diameter (Id) =	6.03	FT.
Outside Bse. Flng. Diam. (Od) =	6.69	FT.
Diam. of Tower Wall (Dwall) =	6.69	
Base Shear (Fx) =	18,078	LBS.
Dead Load (Fz) =	14,900	LBS.
Maximum Moment (M) =	1,284,000	FT.-LBS.
Length of Foundation (L) =	18.0	FT.
Area of Long Bolts (abl) =	0.606	IN.^2
Embedment Plate Width (Pw) =	0.000	FT.
Tnsl. Strngth. Twr. Bs. Plt. (fyb) =	36,000	PSI.
Concrete Cmprsv. Strngth (fc) =	4,000	PSI.
Density of Concrete (lc) =	150	Lbs./Ft.^3
Cncrt. Modulus of Elstcty. (Ec) =	3,604,997	PSI.
Steel Modulus of Elasticity (Es) =	29,000,000	PSI.
Width of Bs. Flng. on Twr. (lbf) =	4.0	IN.
Inside Width (lbf) =	5.535	IN.
Diam. of Long Bolts (dbl) =	1.00	IN.
Ultm. Strs. of Long Blts. (ful) =	120,000	PSI.
Ultm. Strngth. of Long Blts. (Ful) =	72,720	LBF.
Number of Long Bolts (nbl) =	30	
Post Tnsn. of Long Bolts (Pbl) =	30,000	LBS.
Base Flange Thickness (Bft) =	2.0	IN.
π =	3.141592654	
Tower Tube Thickness(tt)=	0.375	IN.

Geotechnical Data

Friction Factor (μ_g) =	0.50	(Granular)
Friction Factor (μ_c) =	1.0	(Clay)
Upper Soil Conditions (Us) =	8	FT.
Lower Soil Conditions (Ls) = L - Us =	10	FT.
Direct Shear Angle Above Us (ϕ_s) =	30	Degrees
Direct Shear Angle for Ls (ϕ_L) =	32	Degrees
Soil Density Above Us (λ_s) =	105	Lbs./cu.ft.
Soil Density for Ls (λ_L) =	110	Lbs./cu.ft.
Cohesion Above Us (Cs) =	150	Lbs./sq.ft.
Cohesion for Ls (CL) =	150	Lbs./sq.ft.
Density of Soil Backfill (λ_b) =	100	Lbs./cu.ft.

V = Concrete Volume =	34.44	CY.
Safety Factor (SF) =	3.60	

Dimnsions of Sq. Excavatin Uppr Soils (Deu) =	10.00	Ft.
Dimension orf Sq. Excvatin Lower Soils (Del) =	9.00	Ft.
Average Density of Backfill (λ_a) =	150	Lbs/cu.ft.
Dimnsions of Sq. Excvatin Mid Soils (Dem) =	9.00	Ft.
Height Upper Slurry (Hsu) =	18	Ft.
Height Mid Soil (Hms) =	9	Ft.



Copyright © 2005 by Patrick & Henderson, Inc.

IMPORTANT NOTE :

The information, data and formulas in these calculations is proprietary and is not to be used, reproduced, or copied without the expressed written consent of Patrick and Henderson, Inc.

LATERAL EARTH PRESSURE ANALYSIS / FOUNDATION RESISTANCE TO OVERTURNING

A. Equivalent Fluid Pressures (pcf)

$$\lambda's = \lambda_s[(1 + \sin\phi_s)/(1 - \sin\phi_s)]$$

$$\lambda's = 315$$

$$\lambda'L = \lambda_s L[(1 + \sin\phi_s)/(1 - \sin\phi_s)]$$

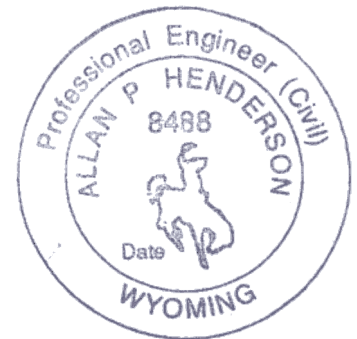
$$\lambda'L = 358$$

B. Passive Resistance Soil Pressures (psf)

1. $f_1 = 2C_s$
 $f_1 =$
2. $f'_1 = f_1 + \lambda's U_s$
 $f'_1 = 2,820.00$
3. $f_2 = 2CL + \lambda's U_s$
 $f_2 = 2,820.00$
4. $f'_2 = f_2 + \lambda'L (z - U_s)$
 $f'_2 = 358.00 \quad -44.04$
5. $f_3 = f_2 + \lambda'L (L - z)$
 $f_3 = 6,400.05$

C. Lateral Forces (Lbf)

1. $S_u = [(f_1 + f'_1)/2] [(H_{su})(De_u) + (H_{ms})(De_m)]$
 $S_u = 407,160.00$
2. $SL = (f_2 + f'_2)/2 (z - U_s) De$
 $SL = 1,790.02 \quad z^2 \quad + \quad -440.38 \quad z \quad 111,038.49$
3. $R = (f_2 + f_3)/2 (L - z) De_l$
 $R = -1,611.02 \quad z^2 \quad 396.34 \quad z \quad 514,836.76$
4. $Ff = mc [Fz + (De_l^2 - PI/4 Do^2) L \lambda_a + PI/4 (Do^2 - Di^2) L \lambda_c + (PI/4 Di^2 lbL)]$
 $Ff = 233,600.00$
5. $F_x = \text{Base Shear}$
 $F_x = 18,078$



D. Sum of Lateral Forces = 0

1. $S_u + SL - R - Ff - F_x = 0$
 $3,401.04 \quad z^2 \quad -836.72 \quad 470,393.26$
 $1.00 \quad z^2 \quad -0.25 \quad 138.31$
 $z = 11.8841$
2. Check Sum of Lateral Forces
 $407,160 \quad 136,537 \quad -737,655$
 $233,600 \quad 18,078 \quad -445,636$

E. Sum of Moments About Point of Rotation "A" (FT-Lbf)

1. $a_1 = z - [(U_s/3)((2f_1 + f'_1)/(f_1 + f'_1))]$
 $a_1 = 6.8072$
2. $a_2 = (z - U_s)[(1 - ((2f_2 + f_2)/(3(f_2 + f'_2)))]$
 $a_2 = 1.8140$
3. $a_3 = ((L - z)/3)((2f_3 + f_2)/(f_2 + f_3))$
 $a_3 = 3.2683$
4. Resisting Moment to Overturn = $Mrt = a_1 S_u + a_2 SL + a_3 R + (L-z) Ff$
 $Mrt = 5,402,367$
5. Overturning Moment at Point of Rotation "A" = $M_o = F_x((M/F_x) + z)$
 $M_o = 1,498,841$
6. Safety Factor to Overturn = $SF = Mrt/M_o$

Copyright © 2005 by Patrick & Henderson, Inc.

IMPORTANT NOTE :

The information, data and formulas in these calculations is proprietary and is not to be used, reproduced, or copied without the expressed written consent of Patrick and Henderson, Inc.

$$SF = 3.60$$

II. STEEL / CONCRETE ANALYSIS

External Load Including 360° Basis (Lbf)

$$F_{ext} = ((4M)/(D_{wall})) - F_z$$

$$F_{ext} = 752,813 \text{ Lbf.}$$

B. Stiffness Constants (Lbf./ft.)

1. Long Bolts

$$C_{bl} = ((n_{bl})(E_s)(a_{bl}))/L$$

$$C_{bl} = 29,290,000$$

2. Lower Concrete

$$C_{cl} = ((\pi/4)(D_o^2 - D_i^2)(E_c))/L$$

$$C_{cl} = 1,449,655,120$$

3. Outer Steel Pipe

$$C_{po} = (E_s)(p)(D_o)(b)(t))/L$$

$$C_{po} = 27,782,770,974$$

4. Inner Steel Pipe

$$C_{pi} = (E_s)(\pi)(D_i)(b)(t))/L$$

$$C_{pi} = 0$$

C. Elongation of Foundation (Ft.)

$$F_{ext} = (C_{bl} + C_{cl} + C_{po} + C_{pi})\Delta u_f$$

$$\Delta u_f = 0.0000 \text{ Ft. (Elongation of Foundation)}$$

D. Tensile Forces in Each Element of Foundation (Lbf.)

$$1. F_{bl} = \text{Force in Long Bolts} = \Delta u_f C_{bl}$$

$$F_{bl} = 754$$

$$2. F_{po} = \text{Force in Outer Steel Pipe} = \Delta u_f C_{po}$$

$$F_{po} = 714,764$$

$$3. F_{pi} = \text{Force in Inner Steel Pipe} = \Delta u_f C_{pi}$$

$$F_{pi} = 0$$

$$4. F_{cl} = \text{Force in Lower Concrete} = \Delta u_f C_{cl}$$

$$F_{cl} = 37,295$$

$$\text{Total} = \underline{\underline{752,813}} = F_{ext \text{ total}} \quad \text{TRUE}$$

$$5. F'^{bl} = \text{Force in Each Long Bolt} = F_{bl}/n_{bl}$$

$$F'^{bl} = 25 \text{ Lbf.}$$

E. Set Compressive Forces in Concrete to Counter Tensile Forces (Lbf.)

1. Set Post Tension in Bolts

a. Long Bolts (P_{bl})

$$F_{cl} = 37,295$$

$$P_{bl} \geq F_{cl}/n_{bl} \text{ (min. post tension)}$$

$$P_{bl} \geq 1,243$$

$$\text{Set } P_{bl} = 30,000 \geq P_{bl} \quad \text{TRUE}$$

$$P'^{bl} = P_{bl} + F'^{bl}$$

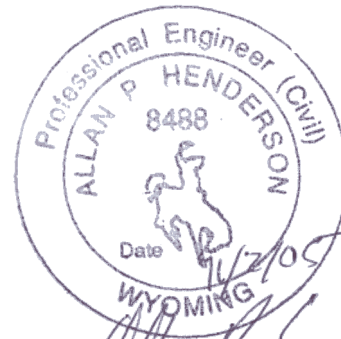
$$P'^{bl} = 30,025$$

$$F_{ybl} = (0.9)(0.8)(F_{ul}) = \text{Max. Allowable Tensile Load}$$

$$F_{ybl} = 52,358$$

$$P'^{bl} < F_{ybl} = \text{TRUE}$$

III. RESISTANCE TO BASE SHEAR (Lbf.)



Copyright © 2005 by Patrick & Henderson, Inc.

IMPORTANT NOTE :

The information, data and formulas in these calculations is proprietary and is not to be used, reproduced, or copied without the expressed written consent of Patrick and Henderson, Inc.

Allowable Shear = $F_v = (0.17)(f_u)(a_b l)(n_b l)$
 $F_v = 370,872$
 $F_x = 18,078$
 $F_v \gg F_x = \text{TRUE}$

IV. CONCRETE COMPRESSION / PULLOUT (PSi.)

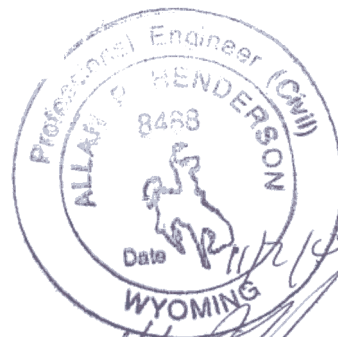
A. Base Flange of Tower

$P_{bf} = \text{Total Load on Base Flange} = P' b l n b l$
 $P_{bf} = 900,754 \quad \text{Lbf}$
 $A_{bf} = \text{Area of Base Flange} = (\pi/4)(O_d^2 - I_d^2)$
 $A_{bf} = 949 \quad \text{In.}^2$
 $A_{ab} = \text{Area of Anchor Bolts} = (a_b l)(n_b l)$
 $A_{ab} = 18.18 \quad \text{In.}^2$
 $f_{cbf} = \text{Concrete Compression at Base Flange} = P_{bf} / (A_{bf} - A_{ab})$
 $f_{cbf} = 967 \quad \text{PSi.}$
 $f_{cbf} < f_c = \text{TRUE}$

B. Bearing Plates - None Required

V. REQUIRED THICKNESS OF BASE FLANGE (In.)

$t_b = \text{Required Thickness} = l b f i (\text{SQRT}(3))(f_{cbf}) / (f_y b * 0.75)$
 $t_b = 1.81 \quad \text{In.}$
 $B_{ft} = \text{Base Flange Thickness Design by Others}$
 $B_{ft} = 2.00$
 $B_{ft} > t_b = \text{TRUE}$



FOUNDATION SUMMARY

Length = 18
 Outside Diameter = 8
 Inside Diameter = 0

 18 LF - 8
 18 LF - 0
 34.44 CY 6 Sack Concrete
 23.94624919 CY 2 Sack Slurry

Copyright © 2005 by Patrick & Henderson, Inc.

8 FT. Diameter CMP
 0 FT. Diameter CMP

IMPORTANT NOTE :

The information, data and formulas in these calculations is proprietary and is not to be used, reproduced, or copied without the expressed written consent of Patrick and Henderson, Inc.